

AS 4060 Supplement 1—1992

**Loads on buried and buried clay
pipes — Commentary**

(Supplement to AS 4060-1992)

This Supplement was prepared by Committee WS/15, Vitrified Clay Pipes. It was approved on behalf of the Council of Standards Australia on 22 July 1992 and published on 16 November 1992.

The following interests are represented on Committee WS/15:

Association of Hydraulic Services Consultants, Australia

Australian Clay Pipe Manufacturers Association

Department of Conservation and Environment, Victoria

Engineering and Water Supply Department, South Australia

Federated Master Plumbers of Australia

Melbourne Water

Public Works Department, New South Wales

Water Authority of Western Australia

Water Board, Sydney — Illawarra — Blue Mountains

Review of Australian Standards. To keep abreast of progress in industry, Australian Standards are subject to periodic review and are kept up to date by the issue of amendments or new editions as necessary. It is important therefore that Standards users ensure that they are in possession of the latest edition, and any amendments thereto.

Full details of all Australian Standards and related publications will be found in the Standards Australia Catalogue of Publications; this information is supplemented each month by the magazine 'The Australian Standard', which subscribing members receive, and which gives details of new publications, new editions and amendments, and of withdrawn Standards.

Suggestions for improvements to Australian Standards, addressed to the head office of Standards Australia, are welcomed. Notification of any inaccuracy or ambiguity found in an Australian Standard should be made without delay in order that the matter may be investigated and appropriate action taken.

This Standard was issued in draft form for comment as DR 91172.

AS 4060 Supplement 1—1992

**Loads on buried and buried clay
pipes — Commentary**

(Supplement to AS 4060-1992)

First published as AS 4060 Supplement 1—1992.

Incorporating:
Amdt 1—1993

PREFACE

This Supplement was prepared by the Standards Australia Committee on Vitrified Clay Pipes as a commentary to provide background information and explanation of the application of AS 4060.

It is not intended that the Standard should be interpreted as preventing the use of methods of load assessment other than those specified, as indeed such alternative methods will possibly be required for circumstances not covered by the Standard. However, it is considered that in the more usually encountered situations, the methods outlined in the Standard are those most acceptable to all concerned, due to their relative simplicity and the length of satisfactory experience so far obtained in their application.

The paragraph numbers of this Commentary are prefixed with the letter 'C' and refer directly to the respective clause numbers of AS 4060, e.g. Paragraph C6.3.2 refers to Clause 6.3.2. Where there is no commentary to a clause of the Standard, the paragraph number does not appear. Figures and tables are designated 1.1, 1.2, etc., and do not correspond to those of AS 4060.

References noted in the text are listed in Appendix A.

Appendix B contains examples illustrating the application of the Standard to the selection of the appropriate crushing strength class, complying with AS 1754 1991, *Vitrified clay pipes and fittings with flexible joints — Sewer quality*. The examples are in the form of typical selection problems each followed by a worked solution.

Appendix C contains data for description, identification, and classification of soils that meet the definition for select fill (see Clause 4 of AS 4060).

© Copyright — STANDARDS AUSTRALIA

Users of Standards are reminded that copyright subsists in all Standards Australia publications and software. Except where the Copyright Act allows and except where provided for below no publications or software produced by Standards Australia may be reproduced, stored in a retrieval system in any form or transmitted by any means without prior permission in writing from Standards Australia. Permission may be conditional on an appropriate royalty payment. Requests for permission and information on commercial software royalties should be directed to the head office of Standards Australia.

Standards Australia will permit up to 10 percent of the technical content pages of a Standard to be copied for use exclusively in-house by purchasers of the Standard without payment of a royalty or advice to Standards Australia.

Standards Australia will also permit the inclusion of its copyright material in computer software programs for no royalty payment provided such programs are used exclusively in-house by the creators of the programs.

Care should be taken to ensure that material used is from the current edition of the Standard and that it is updated whenever the Standard is amended or revised. The number and date of the Standard should therefore be clearly identified.

The use of material in print form or in computer software programs to be used commercially, with or without payment, or in commercial contracts is subject to the payment of a royalty. This policy may be varied by Standards Australia at any time.

CONTENTS

	<i>Page</i>
FOREWORD	4
GENERAL	
C6	WORKING LOADS
C6.1	Types
C6.2	Data
C6.3	Working loads due to trench fill, embankment fill or in-situ material
C6.3.2	Narrow trench condition
C6.3.3	Wide trench or embankment condition (positive projection)
C6.3.4	Embankment condition (negative projection)
C6.3.5	Settlement ratio
C6.3.7	Multiple pipes
C6.3.8	Jacked or bored pipes
C6.4	Working loads due to superimposed dead loads
C6.5	Working loads due to superimposed live loads
C6.5.1	General
C6.5.2	Superimposed live loads
C8	COMPACTION
C9	SUPPORT TYPES AND BEDDING FACTORS
C9.1	Support types
C9.1.1	General
C9.1.3	Granular or concrete
C9.1.3(a)	Type 0 support
C9.1.3(b)(i)(A)	Type 1 support
C9.1.3(b)(i)(B)	Type 2 support
C9.1.3(c)	Type 3 support
C9.1.3(d)	Type 4 and Type 5 support
C9.1.4	Non-granular
C9.2	Bedding factors
C9.2.1	Granular or concrete
C9.2.3	Jacked or bored pipes
C10	DESIGN CRUSHING LOAD
C11	SELECTION OF CRUSHING STRENGTH CLASS
APPENDIX CA	NON-GRANULAR SUPPORT TYPES
CA1	SCOPE
CA2	COMPACTION
CA3	SUPPORT TYPES AND BEDDING FACTORS
CA3.1	Support types
CA3.1.1	General
CA3.1.2	Type U support
CA3.1.3	Type H1 and Type H2 support
CA3.1.4	Type HS1 and Type HS2 support
CA3.1.5	Type HS1 and Type HS2 support
CA3.2	Bedding factors
CA3.2.1	Values
CA3.2.2	Reduction of values
APPENDICES	
A	REFERENCES
B	EXAMPLES OF THE APPLICATION OF PROVISIONS OF AS 4060
C	DATA FOR DESCRIPTION, IDENTIFICATION AND CLASSIFICATION OF SOILS BY THE UNIFIED SOIL CLASSIFICATION SYSTEM

FOREWORD

Designers should be aware that installed pipes may also be subjected to non-vertical loads of considerable magnitude (e.g. at the toe of a slope). The vertical and horizontal components of such loads should be carefully assessed and appropriate action taken to ensure that they can be resisted by the pipes and adequately transferred to the foundations.

Methods of pipe laying and embedment preparation or other aspects of pipeline design and construction, e.g. trench stops, migration of fines, are not stipulated in the Standard because the continuing and rapid development of specialized earthmoving equipment and procedures would soon render such requirements obsolete. However, in Appendix CA, detailed procedures for the compaction of non-granular bedding and haunch support are outlined, as experience has indicated this to be one of the most difficult tasks in the installation of the relevant support types. Attention is also drawn to Geotechnical Branch Training Manuals* which contain much useful information on the subject.

* GEOTECHNICAL BRANCH. Denver, (Colorado): U.S. Department of the Interior, Bureau of Reclamation, Engineering and Research Centre. *Training Manuals* Nos 1, 2, 3 and 7.

STANDARDS AUSTRALIA

Loads on buried vitrified clay pipes — Commentary
(Supplement 1 to AS 4060—1992)

GENERAL The design principles and parameters used in the Standard are based on —

- (a) the theories and research into related parameters of Marston, Schlick and Spangler on the working loads on pipes due to trench fill, embankment fill and in-situ material for tunnels (Ref. 1);
- (b) research to examine the bedding factors for different support types by the Water Research Centre, UK (Ref. 2) and the Clay Pipe Development Association, UK (Ref. 3); and
- (c) the working loads on pipes due to traffic loading specifications by NAASRA (Ref. 4).

The primary purpose of the Standard is to use recent research along with existing knowledge in relation to bedding factors to develop a design standard that is specific to vitrified clay pipe complying with Ref. 5. Long established and universally used equations for the working loads on pipes have been adopted.

A secondary purpose of the Standard is to reduce the large variety of likely installation conditions and support types to a small number.

A support type with the foundation shaped to fit the outside diameter of the pipes has been omitted, as there has been a definite trend away from its use in Australia.

C6 WORKING LOADS

C6.1 Types The total working load is the summation of the different types of working loads for particular application (see Appendix B).

C6.2 Data

- (b) The assessed unit weight of the trench fill, embankment fill or in-situ material should preferably be known or measured and a design value determined prior to commencing calculations. In most cases there will be change in soil strata both along the route of the pipeline and at different levels down to the proposed pipe crown level. It is recommended that the greatest unit weight found be used, rather than complicate calculations with different unit weights for different lengths of pipeline. In the absence of more specific data, the values given below from Ref. 6 relate to materials (see Figure 8 of AS 4060) and may be used for assessment of working loads —
 - (i) saturated clay (fluid): 21 kN/m³;
 - (ii) normal wet clay: 19 kN/m³;
 - (iii) sandy clay: 18 kN/m³;
 - (iv) clayey sand: 16 kN/m³; and
 - (v) loose granular material: 15 kN/m³.

C6.3 Working loads due to trench fill, embankment fill or in-situ material The various equations given in the Standard for calculating dead loads due to fill materials were derived from load conditions associated with particular configurations of an installation. These configurations have been traditionally classified on the basis of the relative levels of the finished surface and the crown of the pipe with respect to the level of the adjacent existing surface, which can be most clearly seen in a cross-section of an installation (see Figures 2, 3, 4, 6 and 7 of AS 4060). Thus embankment or trench installation conditions are shown as cross-sections where the finished surface is, respectively, above or at the level of the adjacent existing surface, and positive or negative projection conditions shown as cross-sections, where the top of the pipe is, respectively, above or below the level of the existing surface.

C6.3.2 Narrow trench condition The coefficient C_1 obtained from Figure 8 of AS 4060 includes an allowance for the development and continuation of a beneficial friction between the trench wall and the trench fill. If for some reason this friction is decreased or not developed, the load on the pipe will be greater than that given by Equation 6.3.2(1) of AS 4060. Decreased or non-development of this friction may occur if, during construction, withdrawal of a trench wall support system (shoring or sheeting) destroys the contact between the trench walls and the trench fill or if the area is subjected to prolonged heavy traffic vibrations. Designers should therefore take into consideration the construction techniques and regulatory requirements likely to apply in the particular project or location and adopt appropriate bedding factors.

The Clause specifies that the load for the narrow trench condition shall be taken as the lower value derived from either the narrow trench or the wide trench or embankment equations. Generally speaking, as a trench becomes wider, the frictional forces noted above become a decreasing proportion of the total vertical forces acting on the pipe until a width is reached where a wide trench or an embankment condition begins to dominate. The theoretical transition width between the narrow trench and the wide trench or the